



# Seismic Examination Of Multistorey Building With Balanced Column

**H.SAI NAVYA**

M.Tech Student, Dept of Civil, Priyadarshini Institute  
of Technology & Science for Women, Chintalapudi  
Village, Tenali, A.P, India

**R.NAGA LAKSHMI**

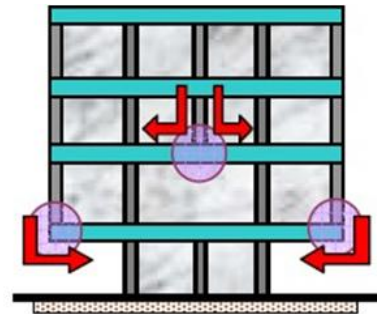
Assistant Professor, Dept of Civil, Priyadarshini  
Institute of Technology & Science for Women,  
Chintalapudi Village, Tenali, A.P, India

**Abstract:** Buildings with telecommunications are a common feature of modern Indian urban planning. These features are not absolutely necessary for a building being built in its surroundings. This study highlights the importance of properly verifying the presence of an ascending column in a home study. Other proposals, including the balance between the former warehouse and the above mentioned, aim to reduce the deficiencies caused by the collapse of the columns. FEM codes for 2D and without moving shafts are developed to study structure response under different earthquakes at different PGA maintenance times and in real time. Usually. The chronological date of floor tiles, middle floor, floor tiles, turnover times, for both sides without columns is calculated.

**Keywords:** Floating Column; Seismic Response; Bracings;

## 1. INTRODUCTION:

Many multi-storey stores in India today opened the store first as an imperative. It is often used in making parking or transportation packages in store first. Although the amount of torque used as a home during an earthquake is dependent on its periodicity, the seismic energy distribution depends on the energy and pressure distribution at high altitude. The behavior of a building during earthquakes depends on its total size, size, and engineering, as well as how the size of the earthquake reached Earth. There is a need to reduce the earthquake forces that have evolved at various points in the building inside the building to higher ground levels in the shortest possible way; i.e. interruptions or interruptions in this interchange path resulted in misuse of the facility. Houses with limited infrastructure (such as hotels with fewer buildings than others) cause a strong earthquake in the event of a contraction. Homes without columns or walls in an unusual store or convenience store are usually damaged or downgraded to start at this store. Many homes that have a tram commercial property fell or badly damaged in Gujarat during the 2001 Bug earthquake [1]. The houses and columns that hang or climb stairs in the store and not all go to the platform, there are some obstacles in the way of transportation. The column should serve as a standoff from the base and begin to send the load to the ground. The term transfer column is also electrically equivalent (due to structural structure / building position) at its lowest level (bottom) above a tree and is a limiting factor. Below are banners at the time the shipment is loaded into other columns.



There are several projects that specify moving columns, especially on the ground floor, which utilize circular motion, so that more space is accessible to the floor. These open spaces may be required for the purpose of the churchyard or park. The transmitted signs should be well-planned and dispersed, especially in earthquake-prone areas. The column is a heavy load on the bike to support it. According to the analysis, it is generally assumed that the column is fixed to the base, and therefore it is considered to be a high point in the transport. STAAD Pro, ETABS and SAP2000 can be used to make predictions of this type of structure [2]. Adjustable valves are strong enough to withstand the pressure of the tire but the transmission should be of sufficient size (viscous) and minimal.



**240 Park Avenue South in New York, United States**



**Palestra in London, United Kingdom**

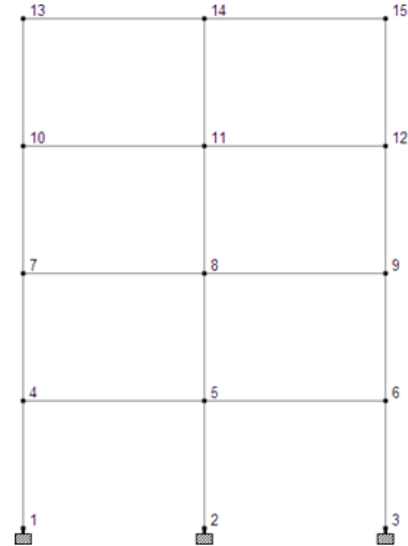
## II. FINITE ELEMENT FORMULATION

The finite element method (FEM), sometimes referred to as the finite element (FEA), is a method that is used to find solutions to different boundary problems in engineering, for example important price problems are also sometimes referred to as cost problems that can be said to be a mathematical problem if it should One or more dependent variables must satisfy a differential equation anywhere within the range of independent variables and also fulfill certain conditions along the boundaries of those address variables [3][4]. Real estate climate problems in FEM often include land as an area of interest that is often represented by a physical structure. Earth adjustments are subject to different boundary variables and values in relation to the specific value of the Earth variables within the field boundary. Field variables may include temperature, temperature, physical variables, and speed of life depending on the type of problem being investigated.

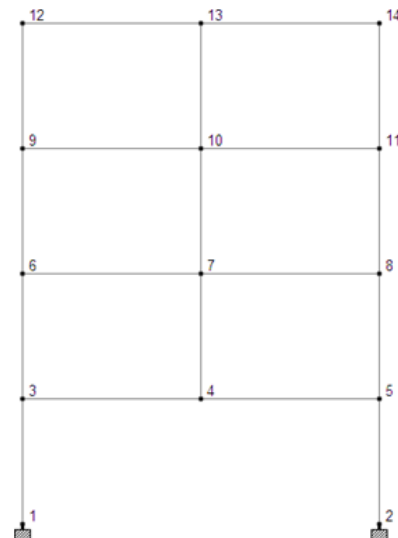
## RESULT AND DISCUSSION

Fig. 1 and Fig.2 Display the graph of two windows without rotating randomly. From Tables 1 and 2, we can observe that the values that are replaced by the current FEM in the form of a frame and a column column are greater than the values of the

displacements in inside the frame without any moving column [5]. Tables 3 and 4 show the nodal replacement cost obtained from the STAAD Pro for the frame without interruption respectively and the result is comparable to the result obtained with the current FEM .



**Fig. 1 2D Frame with usual columns**



**Fig. 2 2D Frame with Floating column**

Node	Horizontal	Vertical	Rotational
	X mm	Y mm	rZ rad
1	0	0	0
2	0	0	0
3	0	0	0
4	1.6	0	0
5	1.6	0	0
6	1.6	0	0
7	3.8	0	0
8	3.8	0	0
9	3.8	0	0
10	5.8	0	0
11	5.8	0	0
12	5.8	0	0
13	6.7	0	0
14	6.7	0	0
15	6.7	0	0

**Table 1 Global deflection at each node Table**

Node	Horizontal	Vertical	Rotational
	X mm	Y mm	rZ rad
1	0	0	0
2	0	0	0
3	0	0	0
4	1.4	0	0
5	1.4	0	0
6	1.4	0	0
7	3.6	0	0
8	3.6	0	0
9	3.6	0	0
10	5.6	0	0
11	5.6	0	0
12	5.6	0	0
13	6.8	0	0
14	6.8	0	0
15	6.8	0	0

**Table 2 Global deflection at each node for general frame obtained for general frame obtained in present FEM in STAAD Pro.**

Node	Horizontal	Vertical	Rotational
	X mm	Y mm	rZ rad
1	0	0	0
2	0	0	0
3	2.6	0	0
4	2.6	0	0
5	2.6	0	0
6	4.8	0	0
7	4.8	0	0
8	4.8	0	0
9	6.8	0	0
10	6.8	0	0
11	6.8	0	0
12	7.8	0	0
13	7.8	0	0
14	7.8	0	0

**Table 3 Global deflection at each node frame with floating column**

Node	Horizontal	Vertical	Rotational
	X mm	Y mm	rZ rad
1	0	0	0
2	0	0	0
3	2.6	0	0
4	2.6	0	0
5	2.6	0	0
6	4.8	0	0
7	4.8	0	0
8	4.8	0	0
9	6.8	0	0
10	6.8	0	0
11	6.8	0	0
12	7.7	0	0
13	7.7	0	0
14	7.7	0	0

**Table 4 Global deflection at each for frame with floating column obtained in present FEM**

### III. FREE VIBRATION ANALYSIS

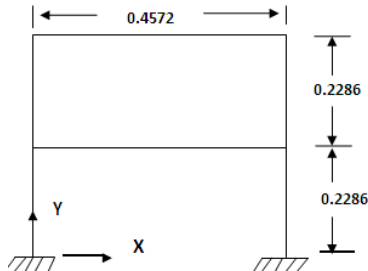
A two storey one bay 2D frame is taken. Fig.4.3 shows the sketchmatic view of the 2D frame [6]. The results obtained are compared with Maurice Pety. The input data are as follows:

Span of bay = 0.4572 m Storey height = 0.2286 m

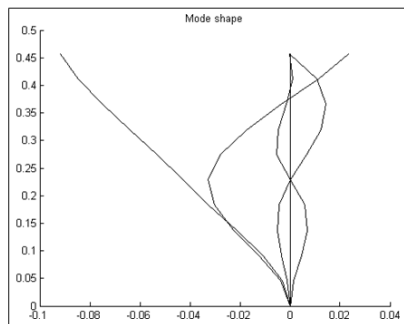
Size of beam = (0.0127 x 0.003175) m Size of column = (0.0127 x 0.003175) m

Modulus of elasticity,  $E = 206.84 \times 10^6 \text{ kN/m}^2$

Density,  $\rho = 7.83 \times 10^3 \text{ Kg/m}^3$



**Fig. 3 Geometry of the 2 dimensional framework.**  
Dimensions are in meter



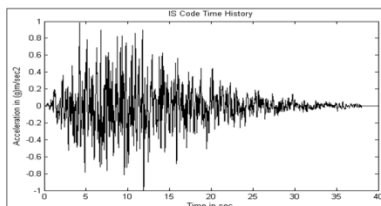
**Fig. 4 Mode shape of the 2D framework**

#### IV. FORCED VIBRATION ANALYSIS

The dimension and material properties of the frame is as follows:

Young's modulus,  $E = 206.84 \times 10^6 \text{ kN/m}^2$

Density,  $\rho = 7.83 \times 10^3 \text{ Kg/m}^3$  Size of beam = (0.1 x 0.15) m Size of column = (0.1 x 0.125) m



**Fig. 5 Compatible time history as per spectra of IS 1893 (part 1): 2002**

#### V. CONCLUSION

Building behavior is examined with and without the use of a shaft under earthquake pressure. The geological time and history of the Elcentro earthquake were evaluated. The PGA of the

earthquake is estimated at 0.2g and the duration of the interest remains unchanged. An example was developed as an element to study multiple account growth behavior. High scores and null scores are used to describe the finite element model. Body durability is studied by changing the column number. We concluded that as the floor area of the upper floor increases, the costs of interior decoration decrease. The initial stage of your hair and scalp varies with the change in column arrangement.

#### REFERENCES:

- [1]. Agarwal Pankaj, Shrikhande Manish (2009), "Earthquake resistant design of structures", PHI learning private limited, New Delhi.
- [2]. Arlekar Jaswant N, Jain Sudhir K. and Murty C.V.R, (1997), "Seismic Response of RC Frame Buildings with Soft First Storeys". Proceedings of the CBRI Golden Jubilee Conference on Natural Hazards in Urban Habitat, 1997, New Delhi.
- [3]. Awkar J. C. and Lui E.M, "Seismic analysis and response of multistory semirigid frames", Journal of Engineering Structures, Volume 21, Issue 5, Page no: 425-442, 1997.
- [4]. Balsamoa A, Colombo A, Manfredi G, Negro P & Prota P (2005), "Seismic behavior of a full-scale RC frame repaired using CFRP laminates". Engineering Structures 27 (2005) 769– 780.
- [5]. Bardakis V.G., Dritsos S.E. (2007), "Evaluating assumptions for seismic assessment of existing buildings "Soil Dynamics and Earthquake Engineering 27 (2007) 223–233.
- [6]. Brodericka B.M., Elghazouli A.Y. and Goggins J, "Earthquake testing and response analysis of concentrically-braced sub-frames", Journal of Constructional Steel Research ,Volume 64, Issue 9, Page no: 997-1007,2008.